Uplift Load Case		Net Uplift Formula, <i>U</i> (N)	Allowable Anchor Bolt or Anchor Strap Stress (MPa)	Allowable Shell Stress at Anchor Attachment (MPa)			
Design Pressure		$[P_i \times D^2 \times 785] - W_1$	$^{5/12} \times F_y$	$^{2}/_{3}F_{ty}$			
Test Pressure		$[P_t \times D^2 \times 785] - W_3$	$^{5/9} \times F_y$	$5/6 F_{ty}$			
Wind Load		$P_{\rm WR} \times D^2 \times 785 + [4 \times M_{\rm WS}/D] - W_2$	$0.8 \times F_y$	$5/6 F_{ty}$			
Seismic Load		$[4 \times M_{rw}/D] - W_2 (1 - 0.4A_V)$	$0.8 \times F_y$	$5/6 F_{ty}$			
Design Pressure <sup>a</sup> + Wind		$[F_p(P_i + P_{WR}) \times D^2 \times 785] + [4 M_{WS}/D] - W_1$	$^{5/9} \times F_y$	$5/6 F_{ty}$			
Design Pressure <sup>a</sup> + Seismic		$[F_p P_i \times D^2 \times 785] + [4 M_{rw}/D] - W_1 (1 - 0.4A_V)$	$0.8 \times F_y$	$5/6 F_{ty}$			
Frangibility Pressure <sup>b</sup>		$[3 \times P_f \times D^2 \times 785] - W_3$	$F_y$	F <sub>ty</sub>			
$A_v$ D $F_p$	is the vertical e is the tank diam is the pressure	al earthquake acceleration coefficient, in % g; iameter, in meters; ure combination factor;					
$F_{tv}$	is the minimum	vield strength of the bottom shell course, in MPa:					
$F_y$	is the minimum strength or 380 exceed the min	num yield strength of the anchor bolt or strap; bolts are limited to specified material minimum yield 380 MPa, whichever is less, in MPa; anchor strap material minimum yield strength shall not minimum yield strength of the shell;					
Н	is the tank heig	ht, in meters;					
$M_{\rm WS}$	equals $P_{\rm WS} \times D$	$D \times H^2/2$ , in N-m;					
$M_{\rm rw}$	is the seismic n	smic moment, in N-m (see Annex E);					
$P_i$	is the design in	design internal pressure, in kPa (see Annex F);					
$P_f$	is the failure pre	failure pressure, in kPa (see Annex F);					
$P_t$	is the test press	ie test pressure, in kPa (see Annex F);					
$P_{\rm WR}$	is the wind uplif	wind uplift pressure on roof, in kPa;					
$P_{\rm WS}$	is the wind pres	nd pressure on shell, in N/m <sup>2</sup> ;					
$W_1$	is the corroded permanent atta	s the corroded weight of the roof plates plus the corroded weight of the shell and any other corroded permanent attachments acting on the shell, in N;					
<i>W</i> <sub>2</sub>	is the corroded portion of the ro	orroded weight of the shell and any corroded permanent attachments acting on the shell including the of the roof plates and framing acting on the shell, in N;					
<i>W</i> <sub>3</sub>	is the nominal attachments ac	is the nominal weight of the roof plates plus the nominal weight of the shell and any other permanent attachments acting on the shell, in N.					
<sup>a</sup> Refer to	5.2.2 concerning the	pressure combination factor applied to the design pressur	re.				
<sup>b</sup> Frangibi	ility pressure applies	only to tanks designed to 5.10.2.6 d.					

Table 5.21a—Uplift Loads (SI)

14	Uplift	Load Case	Net Uplift Formula, U (lbf)	Allowable Anchor Bolt or Anchor Strap Stress (Ibf/in. <sup>2</sup> )	Allowable Shell Stress at Anchor Attachment (Ibf/in. <sup>2</sup> )				
15	Design Pressure		$[P_i \times D^2 \times 4.08] - W_1$	$^{5/12} \times F_y$	$^{2/3} F_{ty}$				
15	Test Pressure		$[P_t \times D^2 \times 4.08] - W_3$	$^{5/9} \times F_y$	<sup>5</sup> /6 F <sub>ty</sub>				
15	Wind Load		$P_{\rm WR} \times D^2 \times 4.08 + [4 \times M_{\rm WS}/D] - W_2$	$0.8 \times F_y$	<sup>5</sup> /6 F <sub>ty</sub>				
I	Seismic Load		$[4 \times M_{\rm rw}/D] - W_2 (1 - 0.4A_V)$	$0.8 \times F_y$	<sup>5</sup> /6 F <sub>ty</sub>				
15	Design Pressure <sup>a</sup> + Wind		$[(F_p(P_i + P_{WR}) \times D^2 \times 4.08] + [4 M_{WS}/D] - W_1$	$^{5/9} \times F_y$	<sup>5</sup> /6 F <sub>ty</sub>				
15	Design Pressure <sup>a</sup> + Seismic		$[F_p P_i \times D^2 \times 4.08] + [4 M_{\rm rw}/D] - W_1 (1 - 0.4A_V)$	$0.8 \times F_y$	<sup>5</sup> /6 F <sub>ty</sub>				
15	Frangibility Pressure <sup>b</sup>		$[3 \times P_f \times D^2 \times 4.08] - W_3$	Fy	F <sub>ty</sub>				
14 15 14 15	$A_{v}$ $D$ $F_{p}$ $F_{ty}$ $F_{y}$ $H$ $M_{WS}$ $M_{rw}$ $P_{i}$ $P_{f}$	is the vertical e is the tank diam is the pressure is the minimum is the minimum strength or 55, minimum yield is the tank heig equals $P_{WS} \times L$ is the seismic r is the design in is the failure pr	the vertical earthquake acceleration coefficient, in % g; the tank diameter, in feet; the pressure combination factor; the minimum yield strength of the bottom shell course, in psi; the minimum yield strength of the anchor bolt or strap; bolts are limited to specified material minimum yield rength or 55,000 psi, whichever is less, in psi; anchor strap material minimum yield strength shall not exceed the inimum yield strength of the shell; the tank height, in feet; puals $P_{WS} \times D \times H^2/2$ , in ft-lbs; the seismic moment, in ft-lbs (see Annex E); the design internal pressure, in inches of water column (see Annex F); the foilure pressure, in inches of water column (see Annex F);						
14	$P_t$ $P_{WR}$ $P_{WS}$ $W_1$ $W_2$ $W_3$	$P_t$ is the test pressure, in inches of water column (see Annex F); $P_{\rm WR}$ is the wind uplift pressure on roof, in inches of water column; $P_{\rm WS}$ is the wind pressure on shell, in lbs/ft <sup>2</sup> ; $W_1$ is the corroded weight of the roof plates plus the corroded weight of the shell and any other corroded permanent attachments acting on the shell, in lbf; $W_2$ is the corroded weight of the shell and any corroded permanent attachments acting on the shell including the portion of the roof plates and framing acting on the shell, in lbf; $W_3$ is the nominal weight of the roof plates plus the nominal weight of the shell and any other permanent attachments acting on the shell, in lbf;							
	a Refer to s	<sup>a</sup> Refer to 5.2.2 concerning the pressure combination factor applied to the design pressure.							
	<sup>b</sup> Frangibility pressure applies only to tanks designed to 5.10.2.6 d.								

## Table 5.21b—Uplift Loads (USC)